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FORCES ON A HORIZONTAL CYLINDER DUE TO NON-LINEAR WAVES

Fred Herman Gehrman, Jr.

Naval Postgraduate School Monterey, California

December 1972

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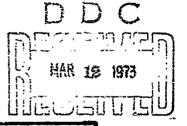


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## **THESIS**



FORCES ON A HORIZONTAL CYLINDER
DUE TO NON-LINEAR WAVES

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Fred Herman Gehrman Jr.

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Thesis Advisor:

C.J. Garrison

December 1972

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# Forces on a Horizontal Cylinder Due to Non-Linear Waves

by

Fred Herman Gehrman, Jr. Lieutenant, United States Navy B. S., The Creighton University, 1966

Submitted in partial fulfillment of the requirements for the degree of

#### MASTER OF SCIENCE IN MECHANICAL ENGINEERING

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Author	F. H. Gehmon Jr.	
Approved by:	C.J. Harrison	
	Thesis /	dvisor
	Robert A Num	
_	Chairman, Department of Mechanical Engir	neering
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#### ABSTRACT

A horizontal cylinder, located near the floor of a two dimensional wave channel was subjected to a train of non-linear gravity waves. The horizontal and vertical forces were measured and presented in dimensionless form. Experimental values of horizontal and vertical force coefficients are presented as functions of dimensionless wave height and dimensionless wave period. The dimensionless force coefficients predicted by a modified Morrison equation and a Froude-Krilov force are compared to experimental data. Fluid particle velocity and acceleration values were calculated from Stokes fifth-order gravity wave theory. Experimental dimensionless wave periods from 10 to 200 were investigated.

The horizontal force coefficients were found to vary linearly with wave height for dimensionless period values from 60 to 120. The vertical force coefficients were found to be inertia dominated at low dimensionless wave heights and dominated by a lift force at higher wave heights. The theory predicted experimental values of force coefficients with good accuracy, especially at greater water depths.

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#### LIST OF SYMBOLS

SYMBOL	DEFINITION	UNITS
a	cylinder radius	ft.
a <sub>x</sub>	horizontal acceleration	ft./sec. <sup>2</sup>
ã <sub>x</sub>	dimensionless horizontal acceleration	
a <sub>y</sub>	vertical acceleration	ft./sec. <sup>2</sup>
a <sub>y</sub> ã <sub>y</sub>	dimensionless vertical acceleration	
C	wave celerity	ft./sec.
ĉ	dimensionless wave celerity	
c <sub>D</sub>	drag coefficient	
$\mathbf{c}_\mathtt{L}$	lift coefficient	
c <sub>M</sub>	added mass coefficient	
ă	depth to wave length ratio	
F <sub>x</sub>	horizontal force component	lbf.
₹ x	dimensionless horizontal force component	
F <sub>y</sub>	vertical force component	lbf.
řy	dimensionless vertical force component	
F <sub>i</sub>	vertical inertial force	lbf.
ř <sub>i</sub>	dimensionless vertical inertial force	
F <sub>L</sub>	vertical lift force	lbf.
$\mathbf{ ilde{P}_L}$	dimensionless lift force	
g	gravitational constant	ft./sec. <sup>2</sup>

SYMBOL	DEFINITION	UNITS
gT <sup>2</sup> /h	dimensionless period parameter	
h	water depth	ft.
ĥ	dimensionless water depth	
H	peak to trough wave height	ft.
ñ	-wave height to diameter ratio, dimensionless wave height	
Ħ <b>ā</b>	wave height to depth ratio	
L	wave length	ft.
£	cylinder length	ft.
P	pressure	lbf./ft.2
P	dimensionless pressure	
S	position measured from bottom	ft.
u	horizontal velocity	ft./sec.
ũ	dimensionless horizontal velocity	
•	vertical velocity	ft./sec.
7	dimensionless vertical velocity	
y	distance from mean water line	ft.
ÿ	dimensionless distance from mean water line	
P	·fluid density	lbm./ft.3
•	velocity potential	ft. <sup>2</sup> /sec.
Ť	dimensionless velocity potential	

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SYMBOL	DEPINITION	UNITS
λ	arbitrary constant in Stokes Wave Theory	
η	distance from mean water line to free surface	ft.
в	phase angle	degrees

#### **ACKNOWLEDGEMENTS**

The author wishes to express his appreciation, first of all to his advisor, Professor C. J. Garrison, for his enlightenment, encouragement and patience throughout this study. In addition, special thanks is also given to Mr. Tom Christian, who designed the wave height probe without which this study could not have been completed. Finally and most deeply, to Claire, Chris and Karen, for their patience, understanding and long-suffering during this undertaking, the author expresses his love.

#### I. INTRODUCTION

Exploration for and use of ocean resources has received increasing attention in the scientific community in recent years. A considerable effort has resulted in the development of technology in areas such as off-shore oil exploration, recovery of petroleum and deployment of underwater habitats. Oil production often involved the use of very large submerged oil storage tanks and associated piping systems. In some locations, oil is transferred long distances in submerged piping systems, often leading through the surf zone to facilities ashore. Also, waste disposal systems usually involve deployment of large diameter outfall pipes which are laid through the surf zone. Proposals for large scale mining and food gathering activities indicate the trend toward design and construction of large submerged structures.

This activity has generated considerable interest in the interaction of gravity waves with submerged objects and particularly in the forces induced. Most previous investigations [1,2,3] in this area center on the use of a so-called Morrison Equation [3], which involves both a drag and inertial component of force. Unfortunately, the drag and added mass coefficients are found to be neither constant nor simply shape - or Reynolds number - dependent in the unsteady flow that occurs in the interaction of waves with fixed bodies. In addition, where the amplitude of wave motion is large compared to the body dimensions, flow separation occurs,

and the coefficients vary as the flow in a given direction develops. Finally, for small depth to characteristic dimension ratios, the proximity of the free surface may influence the value of the coefficients, even if no flow separation occurs.

In spite of these difficulties, the Morrison Equation will, with certain restrictions, provide a complete description of the horizontal forces acting on a large horizontal cylinder in contact with a plane boundary. If the cylinder is large in comparison to the amplitude of the fluid motion, separation does not occur, and the resulting wave-induced oscillatory flow may be considered an unseparated potential flow about the cylinder. This has been verified by Kuelegan and Carpenter [2] and by Sarpkaya and Garrison [4]. For such cases, the drag component of force is negligible, while the added mass coefficient is a constant.

Application of the Morrison Equation to determine the vertical components of force for a bottom-mounted cylinder ylelds a vertical force component of zero, an obviously invalid result. It is recognized that the vertical force should be composed of two components; one associated with the inertia effects and a second which accounts for the lift force caused by the increased velocity and hence decreased the pressure over the top of the cylinder.

The inertial component of vertical force can be approximated by what is commonly known as a Froude-Krilov force.

This is the force caused by the pressure distribution around

the surface of the cylinder existing in the wave if the cylinder was not present. This is an approximation which does not account for the presence of the body and is therefore considered to be an underestimate of the force.

The lift component of vertical force can be approximated by use of the unseparated, potential flow model used in the analysis of the horizontal forces, i.e., that of a uniform flow past a cylinder in contact with a rigid boundary. The lift coefficient for this case has been determined by Dalton and Helfenstein [5].

A study carried out by Johnson [6] for horizontal forces acting on a bottom-mounted, horizontal cylinder in long waves, assumed that at long wave lengths, the horizontal forces depended on wave height and water depth only. This assumption is, however, of doubtful validity ir the range of Johnson's test.

Shiller [7] measured wave forces on a submerged horizontal cylinder due to small amplitude waves. Over the range of wave heights considered, the magnitude of the horizontal force was found to vary linearly with wave height. The magnitude of the vertical force was found to increase in proportion to the wave height squared.

Perkinson [8] extended Shiller's investigation to include larger wave heights and periods. His study indicates that horizontal forces increased linearly with wave heights and becomes independent of wave period for large wave periods. The vertical forces were found to contain two regimes: the

lower wave lengths where lift force predominates; and the higher wave lengths where inertial force is dominant.

In order to evaluate any forces using the above analyses, it is necessary to first evaluate fluid velocities, accelerations and pressure by use of some wave theory. Fairly simple expressions may be derived for the forces by use of Airy wave theory. However, for finite amplitude waves in finite depth water, a non-linear wave theory is considered to be necessary. Stokes fifth-order wave theory has been chosen for this study.

Stokes originally developed a second-order theory [9] for the case of a non-linear wave in water of finite depth. This method has been extended by Borgman and Chappelear [10] to third order. Skjelbreia and Hendrickson [11] have extended the solution to third and fifth order. Bretschneider [12] has presented a method for extension to any order.

The purpose of this study is to compare the analytical results of Stoke's fifth-order wave theory with experimental values at longer wave periods. The transition section of the wave channel used by Perkinson was extended approximately forty feet to provide a slope of 1:20. This provided the possibility of extending Perkinson's work to include longer wave periods. This study, then, is an extension of the work of Shiller [7] and Perkinson [8].

#### II. THEORETICAL ANALYSIS

#### A. PROBLEM DEFINITION

The problem under consideration is illustrated in Figure 1. A train of gravity waves is considered to progress in the positive x-direction in water of depth h, with rate of propagation C. The fluid particle velocity is expressed in terms of the horizontal and vertical components, u and v. It is of primary interest to determine the horizontal and vertical components of wave force acting on the horizontal cylinder which is in contact with the rigid bottom.

#### B. DIMENSIONAL ANALYSIS

An exact analytical solution to this problem is quite formidable and, therefore, an approximate solution is considered in this work. However, first it is instructive to carry out a dimensional analysis of all pertinent parameters involved.

In general, the maximum of the wave force per unit length acting on a submerged cylinder due to wave motion is known a priori to depend on the following parameters:

$$F_{\text{max}}/\ell = f_1(h, H, T, L, a, \rho, g, \mu)$$
 (1)

where

F = wave force

h = water depth

H = wave height

T = wave period

L = wave length

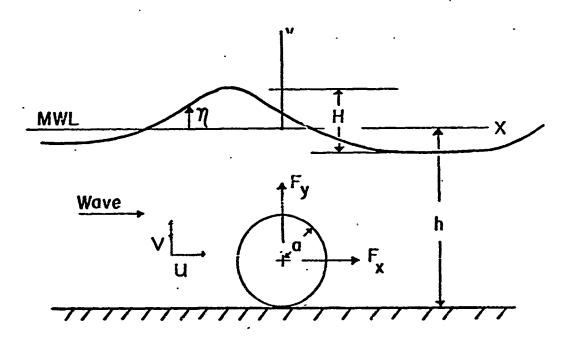


Fig. I. DEFINITION OF GEOMETRY

a = cylinder radius

 $\rho$  = fluid density

g = gravitational constant

 $\mu$  = fluid viscosity

l = cylinder length

However, a relationship exists between the parameters associated with the incident wave (i.e. h,H,T and L).

Consequently, only three of these parameters (e.g. h,H and T) are needed to completely specify the incident wave. A dimensional analysis of the parameters in equation (1) yields the following groups:

$$F_{\text{max}}/\rho ga^2 l = f_2(gT^2/h, h/a, H/2a, \mu/l\sqrt{gha^2})$$
 (2)

The last term on the right side of equation (2) represents the ratio of Froude number to Reynolds number, indicating the ratio of viscous to inertial forces. It is believed that, if this number is small, it may be neglected from further consideration, and equation (2) may be rewritten

$$\tilde{F}_{\text{max}} = f_3(gT^2/h, \tilde{H}, \tilde{h})$$
 (3)

where

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$$\tilde{F}_{\text{max}} = F_{\text{max}}/\rho \text{ga}^2 t$$

 $\tilde{H} = H/2a$ 

 $\tilde{h} = h/a$ 

The approximate analytical approach of this study seeks:

 To develop expressions for the fluid particle motion in the gravity wave as functions of the incident wave parameters, and 2). To develop exprestions for the forces on the submerged cylinder as functions of the fluid particle motion.

The assumptions upon which this analysis is based are:

- 1). Water will be considered inviscid and incompressible,
- 2). The cylinder radius will be considered small in comparison to the dimensions (h and L) of the incident wave.

#### C. WAVE THEORY

The motion of fluid particles in a gravity wave based on assumptions 1) and 2) above is specified by the following boundary value problem.

1). Governing Equation

Assumption 1) implies that Laplace's Equation governs the fluid motion. That is:

$$\nabla^2 \phi = 0 \tag{4a}$$

where  $\phi$  is the velocity potential and the two velocity components are given in terms of  $\phi$  as

$$u = \frac{\partial \phi}{\partial x}$$
,  $v = \frac{\partial \phi}{\partial y}$  (4b)

2). Boundary Conditions

The boundary condition on the bottom is that of a non-porous wall, i.e.,

$$\frac{\partial \phi}{\partial y} = 0 \quad \text{at } y = -h \tag{5}$$

Two boundary conditions are encountered at the water surface, one dynamic and one kinematic.

The dynamic boundary condition is obtained from Bernoulli's Equation by setting the pressure equal to zero. That is:

$$u^2 + v^2 + 2\frac{\partial \phi}{\partial t} = -2g(II - y)$$
 at  $y = \eta$  (6)

where I is the total energy head, a constant.

Assuming the wave travels without change in form, it is then possible to choose a reference system moving in the positive x direction with wave celerity C. This makes the fluid motion steady with respect to the moving reference system. The Bernoulli Equation in such a reference system is:

$$(u - c)^2 + v^2 = 2g(II - y)$$
 at  $y = \eta$  (7)

The kinetic boundary condition imposes the condition that no fluid be transported across the free surface. Or, in other words, the free surface must be a streamline. This may be expressed mathematically in the form:

$$\frac{\partial y}{\partial x} = \frac{y}{u - C} \quad \text{at } y = \eta \tag{8}$$

Equations (4a), (5), (7), and (8) completely specify the boundary value problem.

There are, historically, several approximate solutions to this problem. The solution to be considered further in this analysis is that commonly known as Stokes fifth-order wave theory.

#### D. STOKES FIFTH ORDER SOLUTION

Stokes proposed [9] a power series type of solution to the above problem and proved the validity of the second order solution based on this power series. Skjelbreia and Hendrickson [9] have extended this theory to fifth-order, yielding a solution to the boundary value problem of the form:

$$\phi = \frac{C}{\beta} \{A_1 \cosh(\beta S) \sin \theta + A_2 \cosh(2\beta S) \sin(2\theta) + A_3 \cosh(3\beta S) \sin(3\theta) + A_4 \cosh(4\beta S) \sin(4\theta) + A_5 \cosh(5\beta S) \sin(5\theta)\}$$
(9a)

where  $A_1$  through  $A_5$  are as given in Appendix A.

This equation may be written in dimensionless form as:

$$\tilde{\phi} = \tilde{C}\{A_1 \cosh(\beta S) \sin \theta + A_2 \cosh(2\beta S) \sin(2\theta) + ...\}$$
(9b)
where: 
$$\tilde{\phi} = \beta \phi / \sqrt{gh}$$

$$\tilde{C} = C / \sqrt{gh}$$

Differentiation of equation (9a) with respect to time yields

$$\frac{\partial \phi}{\partial t} = C^2 \{A \cdot \cosh(\beta S) \cos \theta + 2A_2 \cosh(2\beta S) \cos(2\theta) + 3A_3 \cosh(3\beta S) \cos(3\theta) + 4A_4 \cosh(4\beta S) \cos(4\theta) + 5A_5 \cosh(5\beta S) \cos(5\theta) \}$$
 (10a)

or, in dimensionless form

$$\frac{\partial \dot{\phi}}{\partial t} = \tilde{C}^2 \{ A_1 \cosh(\beta S) \cos(\theta) + 2A_2 \cosh(2\beta S) \cos(2\theta) + \dots \}$$
 (10b)

where  $\frac{\partial \phi}{\partial t} = \frac{\partial \phi}{\partial t} / \sqrt{gh}$ 

The series form of the wave profile is assumed to be:

$$y = \frac{1}{\beta} \{ \lambda \cos(\theta) + B_2 \cos(2\theta) + B_3 \cos(3\theta) + B_4 \cos(4\theta) + B_5 \cos(5\theta) \}$$
 (11a)

where  $B_2$  through  $B_5$  are as given in Appendix A and  $\lambda$  is an arbitrary constant.

Equation (lla) can be expressed in dimensionless form as

$$\tilde{y} = \frac{1}{\tilde{d}} \{ \lambda \cos(\theta) + B_2 \cos(2\theta) + \ldots \}$$
 (11b)

where

$$\hat{y} = y/d$$

$$\tilde{d} = 2\pi h/L$$

The series form for I is assumed to be:

$$\pi = \frac{1}{8} \{ \lambda^2 c_3 + \lambda^4 c_4 \} \tag{12}$$

where  $c_3$  and  $c_4$  are as given in Appendix A.

The wave celerity expressed in series form is

$$c = \frac{y}{\beta} c_0^2 \{1 + \lambda^2 c_1 + \lambda^4 c_2\}$$
 (13a)

or, in dimensionless form

$$\tilde{c} = \frac{1}{\tilde{d}} c_0^2 \{ 1 + \lambda^2 c_1 + \lambda^4 c_2 \}$$
 (13b)

where  $C_1$  and  $C_2$  are as given in Appendix A.

Using equations (4b) and (9b) we find the horizontal particle velocity to be:

$$\ddot{u} = (\frac{\partial \phi}{\partial x}) = \tilde{C}\{A_1 \cosh(\beta S) \cos(\theta) + 2A_2 \cosh(2\beta S) \cos(2\theta) + 3A_3 \cosh(3\beta S) \cos(3\theta) + 4A_4 \cosh(4\beta S) \cos(4\theta) + 5A_5 \cosh(5\beta S) \cos(5\theta)\}$$
(14)

where  $\bar{u} = u/\sqrt{gh}$ 

Also, from equations (4b) and (9a):

$$\tilde{\mathbf{v}} = (\frac{\tilde{\mathbf{a}} \hat{\mathbf{b}}}{\tilde{\mathbf{a}} \hat{\mathbf{y}}}) = \tilde{\mathbf{C}} \{ \hat{\mathbf{A}}_1 \sinh(\beta \hat{\mathbf{S}}) \cos(\theta) + 2\hat{\mathbf{A}}_2 \sinh(2\beta \hat{\mathbf{S}}) \cos(2\theta) + 3\hat{\mathbf{A}}_3 \sinh(3\beta \hat{\mathbf{S}}) \cos(3\theta) + 4\hat{\mathbf{A}}_4 \sinh(4\beta \hat{\mathbf{S}}) \cos(4\theta) + 5\hat{\mathbf{A}}_5 \sinh(5\beta \hat{\mathbf{S}}) \cos(5\theta) \}$$
(15)

where  $\tilde{v} = v/\sqrt{gh}$ 

Differentiation of equations (14) and (15) with respect to time yields the particle accelerations

$$\tilde{a}_{x} = (\frac{\partial u}{\partial t}) = \tilde{d}\tilde{c}^{2}\{A_{1}\sinh(\beta S)\sin(\theta) + 2A_{2}\sinh(2\beta S)\sin(2\theta) + 3A_{3}\sinh(3\beta S)\sin(3\theta) + 4A_{4}\sinh(4\beta S)\sin(4\theta) + 5A_{5}\sinh(5\beta S)\sin(5\theta)$$
(16)

where  $\tilde{a}_{x} = a_{x}/g$ 

and

$$\tilde{a}_{y} = \tilde{d}\tilde{C}^{2}\{A_{1}\sinh(\beta S)\cos(\theta) + 2A_{2}\sinh(2\beta S)\cos(2\theta) + ...\}$$
(17)

where  $\tilde{a}_y = a_y/g$ 

The potential function and it's derivatives, as well as the wave profile are now known in terms of the unknown quantities  $\lambda$  and  $\tilde{d}$ . Specifying equations for these parameters are found by developing equations for the peak-to-trough wave height and the wave period. That is, the dimensionless wave height is defined as:

$$\cdot \tilde{H}d = \tilde{y}_{\theta=0} - \tilde{y}_{\theta=0}$$
 (18)

where Hd = H/h

Substitution of equation (13b) into equation (18) yields:

$$\widetilde{H}d = \frac{2}{\widetilde{d}} \{\lambda + \lambda^3 B_{33} + \lambda^5 (B_{35} + B_{55})\}$$
 (19)

where  $B_{33}$ ,  $B_{35}$ , and  $B_{55}$  are as given in Appendix A.

Now, the wave period and the wave celerity are related by the relationship:

$$C = L/T \tag{20}$$

So, equation (15a) becomes

$$(L/T)^2 = g \frac{c_0^2}{g} \{1 + \lambda^2 c_1 + \lambda^4 c_2\}$$
 (21)

or,

$$h/gT^2 = \frac{\tilde{d}}{4T^2} c_0^2 \{1 + \lambda^2 c_1 + \lambda^4 c_2\}$$
 (22)

The parameter gT<sup>2</sup>/h is referred to herein as the period parameter.

Equations (19) and (22), if solved simultaneously, yield values for  $\lambda$  and  $\tilde{d}$  in terms of  $\tilde{Hd}$  and  $gT^2/h$ , quantities which are incident wave parameters. Thus the potential function, particle velocities and accelerations are shown to be functions of the dimensionless parameters obtained by dimensional analysis and given by equation (3). (Note that  $\tilde{Hd} = 2H/h$ )

The next step in the analysis is to express the forces acting on the cylinder as functions of the potential function, particle velocities and accelerations.

#### E. FORCES ON THE CYLINDER

The forces acting on the cylinder are described in terms of their horizontal and vertical components.

#### 1). Lorizontal Component

The horizontal component of force acting on the cylinder is expressed in the form of the so-called Morrison Equation as.

$$F_{x} = \frac{c_{D}}{2}(\rho 2aku^{2}) + (1.0 + c_{M})\pi a^{2}ka_{x}$$
 (23)

where  $C_{D} = drag coefficient$ 

 $C_{M}$  = added mass coefficient

For cases where fluid particle motion is small in comparison with the cylinder diameter, flow separation does not occur and the contribution of drag to the total force may be disregarded. In this case, equation (23) may be rewritten

$$\tilde{\mathbf{F}}_{\mathbf{x}} = (1.0 + \mathbf{c}_{\mathbf{M}})\tilde{\mathbf{a}}_{\mathbf{x}} \tag{24}$$

where  $\tilde{F}_{x} = F_{x}/\rho ga^{2}l$ .

The added mass coefficient for a circular cylinder in contact with a rigid wall was given in closed form by Garrison [13] as  $C_{M} = 2.29$ .

#### 2). Vertical Force Component

As mentioned previously, application of the Morrison Equation to the vertical component of force yields a result which is clearly erroneous. The vertical force may be considered to arise from two sources: an inertial force and a lift force.

#### a). Lift Force

The lift force may be expressed as:

$$F_{L} = 1/2(2al\rho)C_{L}u[u]$$
 (25)

where  $C_{T_i} = 1$ ift coefficient.

or, in dimensionless form:

$$\tilde{F}_{L} = \tilde{h}c_{L}u|u| \qquad (26)$$

where  $\tilde{F}_L = F_L/\rho ga^2 t$ .

Dalton and Helfstein indicate [7] that  ${\rm C_L}$ =4.49 for unseparated flow past a circular cylinder in contact with a rigid wall.

#### b). Inertial Force

The pressure distribution around the surface of the cylinder existing if the cylinder is not present results in a net vertical force (known as a Froude-Krilov force).

Consider the resultant differential force  $dF_1$  acting on a differential area of the cylinder dA as shown in Figure 2. The above equation, written in dimensionless form, is:

$$\tilde{F}_{L} = \int_{0}^{2\pi} \tilde{P}(a,\gamma) \sin\gamma d\gamma \qquad (30)$$

The pressure is obtained from the Bernoulli Equation:

$$P = -\rho \{\frac{1}{2}(u^2 + v^2) + \frac{\partial \phi}{\partial t}\}$$
 (31)

or, in dimensionless form:

$$\tilde{P} = -\tilde{h} \left[ \frac{1}{2} (\tilde{u}^2 + \tilde{v}^2) + (\frac{\tilde{\partial} \phi}{\tilde{\partial} t}) \right]$$
 (32)

where  $\tilde{P} = P/\rho ga$ 

Therefore

$$\tilde{F}_{i} = -\tilde{h}_{0}^{2\pi} \int_{0}^{2\pi} \left[ \frac{1}{2} (\tilde{u}^{2} + \tilde{v}^{2}) + (\frac{\tilde{\partial \phi}}{\tilde{\partial t}}) \right] \sin\gamma dY$$
 (33)

And the total vertical force may be written:

$$\tilde{\mathbf{F}}_{\mathbf{y}} = \tilde{\mathbf{h}}\{\mathbf{c}_{\mathbf{j}}\tilde{\mathbf{u}}|\tilde{\mathbf{u}}| - \int_{0}^{2\pi} \left[\frac{1}{2}(\tilde{\mathbf{u}}^2 + \tilde{\mathbf{v}}^2) + (\frac{\tilde{\mathbf{d}}\phi}{\tilde{\mathbf{d}}t})\right] \sin\gamma d\gamma \}$$
(34)

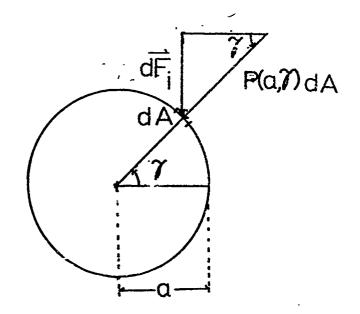


Fig. 2
INERTIAL FORCE GEOMETRY

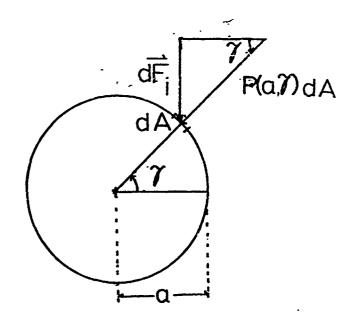


Fig. 2
INERTIAL FORCE GEOMETRY

#### F. DISCUSSION OF COMPUTER PROGRAM

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The equations developed above were programmed for a digital computer as given in Appendix D. The basic inputs to the program are  $\tilde{H}$ ,  $\tilde{h}$  and  $gT^2/h$ . The program yields particle velocity, acceleration and computes the horizontal and vertical forces on the cylinder at various wave angles.

The STAKES5 subroutine calculates the coefficients for the Stokes fifth order equations. Simultaneous solution to equations (19) and (22) are obtained by assuming an initial value of  $\lambda$  and  $\tilde{d}$  based on linear wave theory and iterating to the actual value using the Newton-Raphson technique.

The main program generates tables of values and in-line graphs of the wave profile, particle velocity vs. depth, vertical and horizontal force vs. phase angle.

### III. <u>DESCRIPTION OF APPARATUS AND</u> EXPERIMENTAL PROCEDURE

#### A. WAVE CHANNEL

Wave forces on the cylinder were measured experimentally in a wave channel. The basic wave channel, as shown in Figure 3, was fifteen inches wide and consisted of two sections: a transition section and a shallow water section. The overall channel length was 87 feet.

Three-quarter inch exterior plywood sheets were used for the channel sides. Vertical rigidity was provided by bracing the plywood sheets with 2x4's at various intervals. The vertical members were placed four feet on centers in the shallow water section and from four feet to eighteen inches in the transition section. Additional rigidity was provided in the transition section by truss arrangements, as shown in Figure 4. Two 2x4's were attached to the top of the vertical studs and extended the length of the channel.

A double floor constructed of two plywood sheets with two half-inch separators was used. The sides of the channel were bolted together, through the spacing of the double bottom, with three-eights inch threaded rod.

All wooden sections exposed to water were water-proofed and painted with three coats of Morewear Vitri-Glaz 1320A.

Dow Corning Sealant 780 was applied to all seams and joints.

A paddle type wave generator was used to generate waves in the channel. An aluminum plate was hinged at the tank

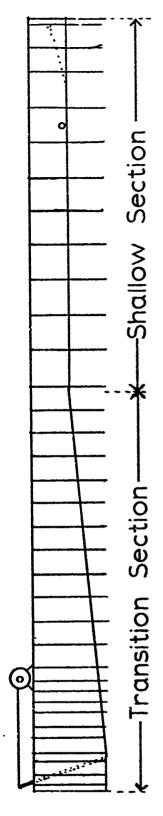


Fig.3 WAVE CHANNEL

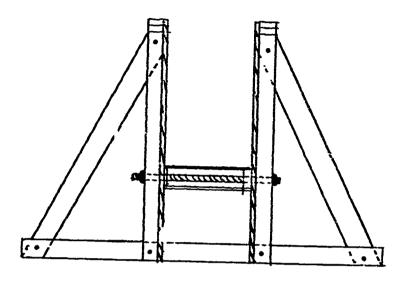


Fig.4

TRANSITION SECTION TRUSS ARRG'T.

floor and attached by a pin connection at the top to a driving rod. Wave motion behind the paddle was damped by use of baffle plates. A two horse power variable speed drive, with an output speed range of twenty to one hundred eighty revolutions per minute, was mounted on top of the wave channel and fitted with a six and three quarter inch radius face plate. Ways were attached to the plate to adjust the driving rod eccentricity between zero and eight inches.

The transition section contained a ramp with a slope of 1:20 which shoaled the generated wave with a change in depth from four to two feet. The overall length of the transition section was 42 feet.

The shallow water section had a depth of two feet and an overall length of 42 feet. The test module was located twenty four feet from the transition, allowing the waves to reach a fully developed state following the transition before reaching the test section.

A variable slope dissipator beach was located at the end of the test section in order to dissipate the wave motion. The beach was constructed of metal shavings held between two pieces of perforated stainless steel sheet metal, which were separated by two inch wooden spacers. A solid sheet of one-eighth inch aluminum was attached to the bottom of the beach by three and five eighths inch separators. One by one inch aluminum angles were attached to the exposed surface of the beach, parallel to the wave fronts. Maximum slope of the beach was 1:7.

#### B. TEST MODULE

The test module consisted of the circular cylinder with its own floor and walls, constructed as an integral system to provide easy removal of the model from the tank. The wave channel sides and floor were recessed to provide a smooth transition from channel to test module. The channel walls were made of plexiglass in the test module region to allow maximum visibility of the model. The test module is shown in Figure 5.

A four inch diameter plexiglass cylinder was supported between the walls of the test module by use of cantilever beams and adjusted to approximately one-sixteenth inch above the floor. A thin flexible plastic barrier was installed in this gap to prevent any water motion under the cylinder due to wave motion. The barrier was held in place by '0' ring material pressed into 0.007 inch slots in the cylinder and test module floor.

The cantilever beams were used to support the cylinder, as shown in Figures 6 and 7. Bulkheads were fixed in the cylinder at approximately two inches from each end, and the fixed ends of the cantilever beams bolted on these bulkheads. The free ends of the two beams protruded slightly beyond the end of the cylinder and into the plexiglass test module walls where they were supported by small self-aligning ball bearings pressed into the plexiglass. Both beams were fitted with strain gauges and waterproofed using BLH Barrier 'C', as shown in Figure 8.

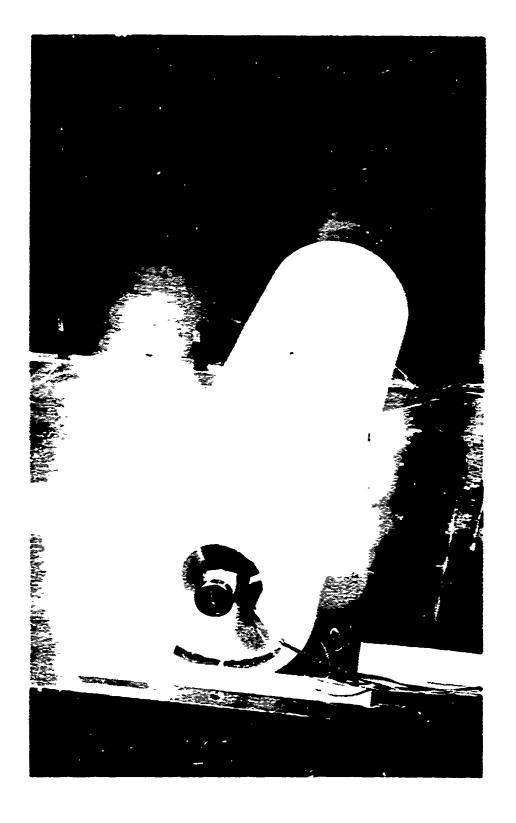


Fig. 5 TEST MODULE

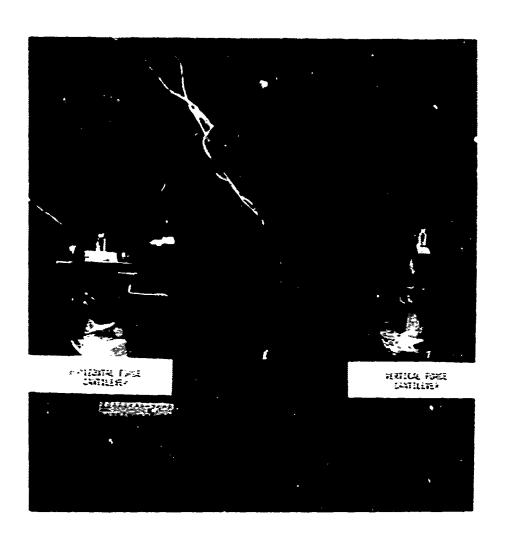


Fig. 6 HORIZONTAL FORCE CANTILEVER



Pig. 7 VERTICAL FORCE CANTILEVER

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Fig. 8 FORCE CANTILEVER

One beam, positioned with its largest cross sectional dimension vertical, was used to measure horizontal force and the other, with its largest cross section positioned horizontally, was used to measure vertical force. Calibration tests indicated that cross coupling was negligible.

Dimensions of the beams were chosen to provide sufficient flexibility to allow measurement of forces using strain gauges and sufficient stiffness to provide a natural frequency that was large in comparison with the excitation frequency. In addition, the minimum width of the beams was constrained by the width necessary to mount the strain gauges. A reasonable trade-off was arrived at by making use of the maximum sensitivity of the amplifier/recorder and determining a maximum allowable value for the cantilezer beam length.

To prevent rotation of the cylinder, an offset arm was used to connect the horizontal force cantilever to the test module. A Farber Bearing, AMS 1K7, was used as a wheel at the test module end of the arm to reduce friction.

#### C. WAVE HEIGHT PROBE

A capacitance type probe was chosen to measure wave heights in this study. The probe essentially consists of a single insulated wire which acted as one plate of a capacitor, the water acting as the second plate. The capacitance varied as the water level rose and fell. A three-eighths inch diameter acrylic rod was attached to the bottom of the foil-like support member and the sensing wire was attached

at the bottom of the acrylic rod and insulated there. The tip of the acrylic rod was removable to provide easy changing of the sensing wire. The wire was connected at the top to a cable connector mounted on a nylon block. Number 30 A.W.G. wire with poly thermaleze insulation was used.

The schematic of the electronic circuit utilized with this probe is shown in Figure 9. In this circuit, the square wave output of a multi-vibrator is used to drive a capacitance bridge, one leg of which is varied by the wave height probe. The resulting AC error signal is converted to a D.C. output signal by the full wave rectifier. The output signal was found to be insensitive to small multivibrator frequency changes and linear through the range of the wave height probe. A model LD 5211-13 Brush D.C. amplifier was used with the probe circuit.

## D. TEST PROCEDURE

Calibration of the probe was conducted with the tank filled to the desired level and the test module and wave height probe in position. With the probe immersed to a depth of nine inches, the amplifier gain was set at desired sensitivity and the recorder pen bias adjusted to position the recorder to mid-scale. The probe immersion was varied over a seven inch range in half inch increments, using a traverse mechanism. This initial calibration was checked at several points before and after each set of runs.

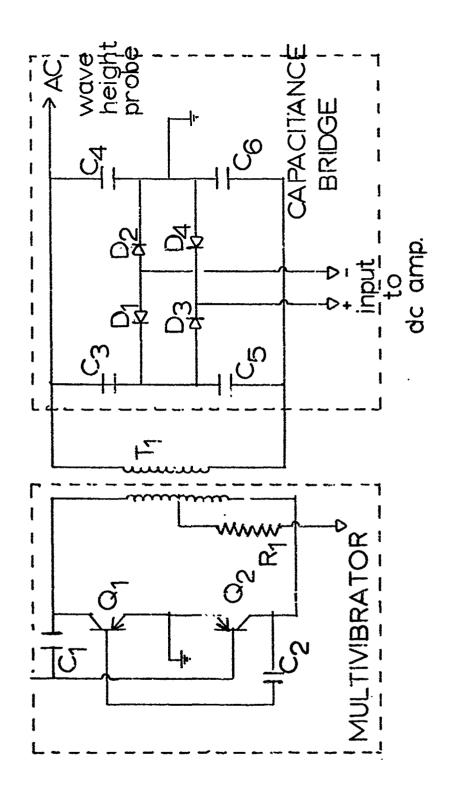


Fig. 9 WAVE HEIGHT PROBE CIRCUIT

The force cantilevers were rated by loading the cylinder with a series of weithts, using a pulley arrangement shown in Figure 10. The load was transmitted to the cylinder through a series of tapped holes located around the circumference at positions horizontally fore and aft, and vertical. The Brush recorder output was calibrated by loading the cylinder in half-pound increments. This calibration was also checked prior to and after each series of runs.

After calibration was completed, the wave generator speed was set for the desired wave length and a data run commenced. During each run, wave generator eccentricity was varied from one inch to eight inches, or to a point where the waves broke in the channel.

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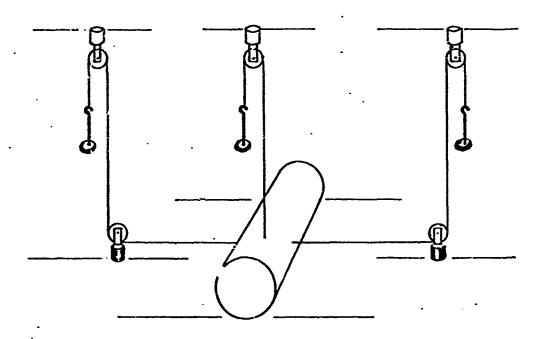


FIGURE 10 CANTILEVER CALIBRATION ARRANGEMENT

# IV. PRESENTATION OF RESULTS AND CONCLUSIONS

As discussed in the theoretical analysis, dimensionless wave force coefficients can be represented as functions of the dimensionless water depth (h/a), the dimensionless wave height (H/2a), and dimensionless period (gT<sup>2</sup>/h) parameters. A test program was developed which, for a given run, held water depth and period constant while varying wave height, in order to determine the effect of the dimensionless parameters on the horizontal and vertical force coefficients. Four series of experimental runs were conducted at relative water depths of 9.0, 7.0, 5.5, 4.0.

The dimensionless wave period parameter was varied for each series over a range of 100 to the maximum value attainable with the exprimental apparatus. The upper limit in wave height was usually provided by breaking.

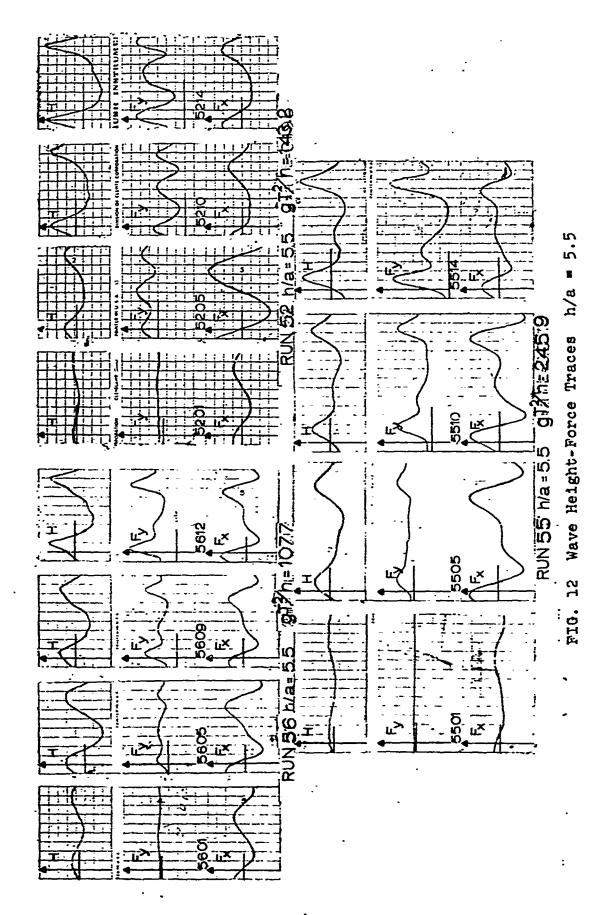
A series of representative traces are shown in Figures 11, 12, 13 and 14 due to the difficulty in presenting all the traces. These traces represent experimental data for three dimensionless periods from each of the four dimensionless water depths. The uppermost trace in each run represents wave height, the middle trace represents vertical force and the bottom trace represents horizontal force. The experimental values of the force coefficients for each experimental case are given in Appendix B.

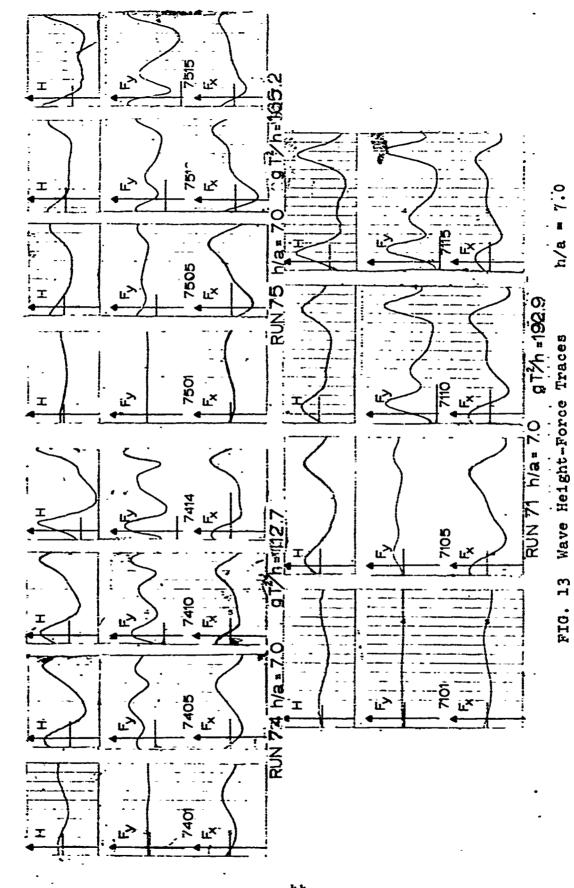
Using the dimensionless period, water depth and waveheight as inputs, the computer program was used to generate

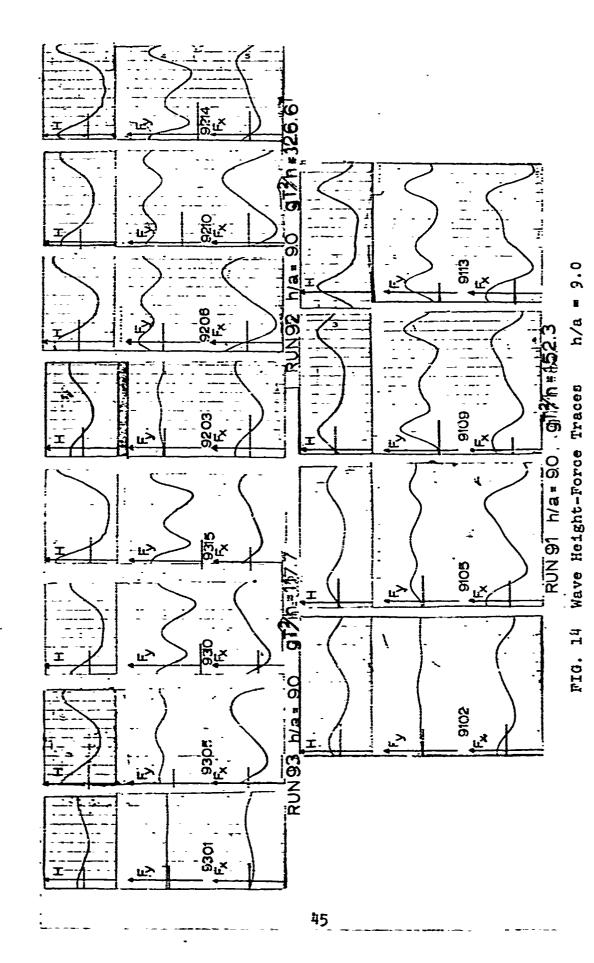
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IG. 11 Wave Height-Force Traces

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corresponding theoretical values of the force coefficients for each experimental case. This provided a comparison between experimental and theoretical values for each run. Plots of experimental and theoretical values for maximum horizontal and vertical force coefficients as a function of wave height for each period and water depth studied are included in Appendix C.

Experimental results and theoretical computations indicate that, at the lower values of dimensionless period and wave height studied, the wave may be characterized as a linear wave. The horizontal force was found to vary sinusoidally, and increased linearly with waveheight. The vertical force varied approximately as a negative cosine function in this region, having its maximum value in the wave trough. The magnitude of the vertical force coefficient was found to vary approximately as the waveheight squared. These characteristics are shown most clearly in runs Number 44 through 47, 71 through 74 and 93 through 95 in Appendix C.

As the relative period and wave height increased, the wave displayed the sharper peaks and longer, shallower troughs that are characteristic of non-linear gravity waves. The horizontal force, proportional to the fluid particle acceleration, tended to reach maximum values closer to the wave peaks, but still remained symmetrical, with respect to the wave crest, as indicated by the theoretical traces. The theoretical analysis predicted a phase lag between the wave crest and the maximum force as small as 24° at the higher wave heights.

The vertical force component began to display positive values occurring at twice the frequency of the wave peaks, as shown in the experimental traces, Figures 11, 12, 13, 14. This is explained by the increasing importance of the lift component of vertical force, which, theoretically, is similar to a cosine squared function. At extremely long periods, the lift force is clearly dominant, and the maximum force no longer occurs in the middle of the wave trough. The maximum vertical force occurs when the cylinder is under the wave crests where the velocity is greatest.

At large periods and wave heights, a point is reached where flow separation effects are no longer negligible. This is seen most clearly in the vertical force component, whose value becomes suddenly lower than the theory predicts, though still varying as the waveheight squared. This sudden drop in lift is apparently caused by a corresponding decrease in the lift coefficient resulting from flow separation. This phenomenon is most clearly evident in the graphical presentation of runs 51 through 56, and 71 through 73 in Appendix C.

# 1). Horizontal Forces

As previously mentioned, the horizontal forces tended to be nearly sinusoidal in nature with a maximum value which is quite linear with wave height. It is therefore possible to characterize the horizontal force coefficient by it's maximum value ( $F_{\max} / \rho ga^2 t$ ). The variation in the horizontal force coefficient with dimensionless wave height is shown graphically in Appendix C.

The experimental values of horizontal force are seen to vary linearly with wave height for relative period parameter values from 100 to 120. For relative period values greater than 120, the force coefficient varies in an increasingly non-linear fashion. This is consistent with the theory, as shown in Figure 15. However, for the range of period parameters studied, the deviation from linearity is relatively small. It is therefore possible to display the experimental data in terms of the slope of the force coefficient versus wave height plots, as shown in Figures 16 and 17.

The theory predicts the maximum values of horizontal force coefficients prior to the wave heights where separation effects become apparent. The discrepancy between theoretical and experimental values is due to variation of the added mass coefficient in a separated flow as well as a net drag force which is no longer zero.

#### 2). Vertical Forces

As indicated previously, the vertical force displays two regimes; one inertia dominated and one lift dominated. The maximum value of the vertical force coefficient (Fy /pga²t) is seen to vary as the wave height squared throughout the range of data taken. This is shown graphically in Appendix C. For higher values of relative wave height, the magnitude of the vertical force coefficients is less than predicted by the theory, due to separation effects, as explained above.

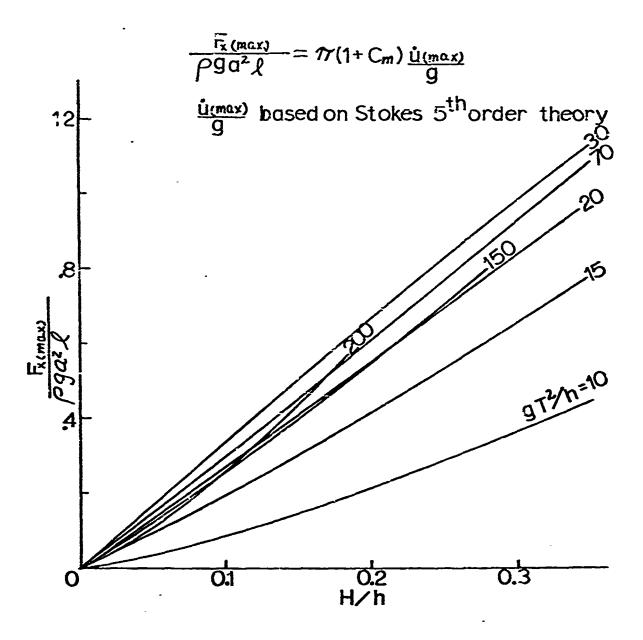
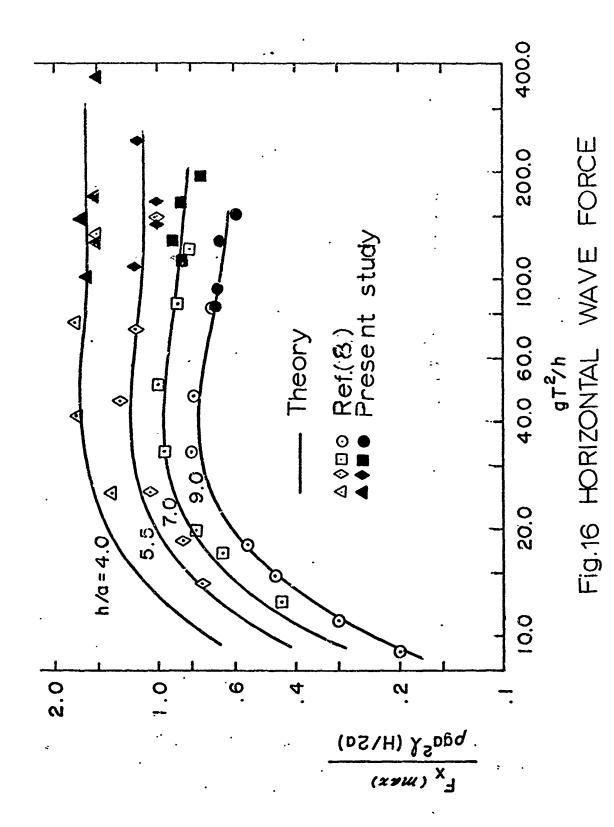


Fig. 15 HORIZ. WAVE FORCE



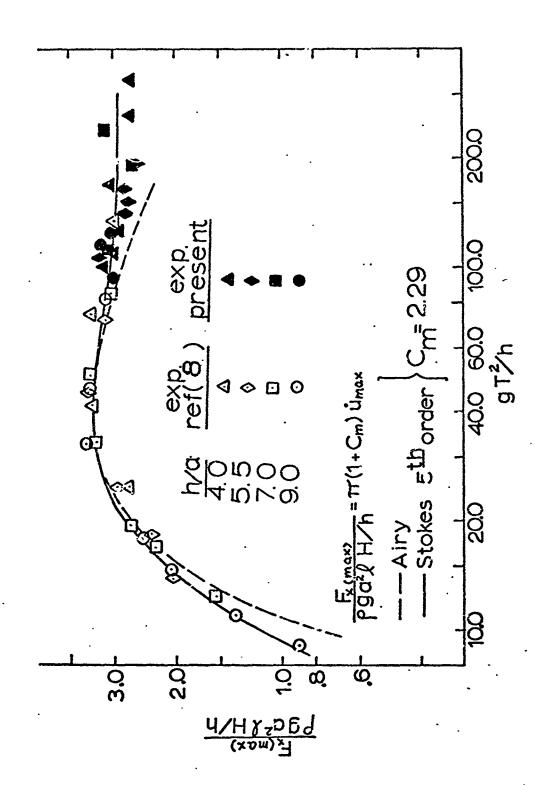


Fig. 17 HORIZON TAL WAVE FORCE

### B. CONCLUSIONS

From the experimental and theoretical results, the following conclusions are considered warranted:

- 1. The horizontal forces on the cylinder, resulting from the incident gravity waves, increased linearly with wave height for relative period values less than 120. For relative period values greater than 120, the force coefficient was found to vary with wave height in an increasingly non-linear fashion.
- 2. The vertical forces displayed two regimes; one inertia dominated at lower periods and wave heights, and one lift dominated at higher values of these parameters.
- 3. For high period parameter values, separation effects can be expected for relative wave height values above 0.6, resulting in a sudden drop in experimental force coefficient values from those predicted by the theory.
- 4. The theory used in this study provides excellent agreement with experimental values for relative period  $(gT^2/h)$  values from 100 to 200, and prior to the onset of flow separation.

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### APPENDIX A

## COEFFICIENTS FOR STOKES FIFTH ORDER WAVE THEORY

$$S = \sinh(\tilde{d})$$

$$C = \cosh(\tilde{d})$$

$$C_0 = \tanh(\tilde{d})$$

$$c_1 = \frac{(8c^4 - 8c^2 + 9)}{8s^4}$$

$$c_2 = \frac{(3840c^{12} - 4096c^{10} + 2592c^8 - 1008c^6 + 5944c^4 - 1830c^2 + 147)}{512s^{10}(6c^2 - 1)}$$

$$c_3 = -\frac{1}{480}$$

$$c_{4} = \frac{(12c^{8} + 36c^{6} - 162c^{4} + 141c^{2} - 27)}{192cs^{9}}$$

$$A_{11} = \frac{1}{S}$$

$$A_{13} = \frac{-c^2(5c^2+1)}{8s^5}$$

$$A_{15} = \frac{-(1184c^{10} - 1440c^8 - 1992c^6 + 2641c^4 - 249c^2 + 18)}{1536s^{11}}$$

$$A_{22} = \frac{3}{8s^4}$$

$$A_{24} = \frac{(1920^8 - 4240^6 - 3120^4 + 4800^2 - 17)}{7685^{10}}$$

$$A_{33} = \frac{(13-4c^2)}{6487}$$

$$A_{35} = \frac{(512c^{12}+4224c^{10}-6800c^8-12,808c^6+16,704c^4-3154c^2+107)}{40968^{13}(6c^2-1)}$$

$$A_{44} = \frac{(80c^6-815c^4+1338c^2-197)}{15368^{10}(6c^2-1)}$$

$$A_{55} = \frac{-(2880c^{10}-72,480c^8+324,000c^6-432,000c^4+163,470c^2-16,245)}{61,4408^{11}(6c^2-1)(8c^4-11c^2+3)}$$

$$B_{22} = \frac{(2c^2+1)}{483}$$

$$B_{24} = \frac{c(272c^8-504c^6-192c^4+322c^2+21)}{38489}$$

$$B_{35} = \frac{(88,128c^{14}-208,224c^{12}+7c,818c^{10}+54,000c^8-21,816c^6+6264c^4-54c^2-81)}{12,2288^{12}(6c^2-1)}$$

$$B_{44} = \frac{c(768c^{10}-448c^8-48c^6+48c^4+106c^2-21)}{38489^9(6c^2-1)}$$

$$B_{55} = \frac{(192,000c^{16}-262,720c^{14}+83,680c^{12}+20,160c^{10}-7280c^8)}{12,2888^{10}(6c^2-1)(8c^4-11c^2+3)} + \frac{(716cc^6-1800c^4-1050c^2+225)}{12,2888^{10}(6c^2-1)(8c^4-11c^2+3)}$$

$$A_1 = \lambda A_{11} + \lambda^3 A_{13} + \lambda^5 A_{15}$$

$$A_2 = \lambda^2 A_{22} + \lambda^4 A_{24}$$

$$A_3 = \lambda^3 A_{33} + \lambda^5 A_{35}$$

$$A_{4} = \lambda^{4}A_{44}$$
 $A_{5} = \lambda^{5}A_{55}$ 
 $B_{2} = \lambda^{2}B_{22} + \lambda^{4}B_{24}$ 
 $B_{3} = \lambda^{3}B_{33} + \lambda^{5}B_{35}$ 
 $B_{4} = \lambda^{4}B_{44}$ 
 $B_{5} = \lambda^{5}B_{55}$ 

# APPENDIX B

# EXPERIMENTAL DATA

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SEGMENT NUMBER	4444444444 WWWWWWWWW 0000000011 WW4W4C

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NON-DIMENSIONAL PERIOD( MGT\*\*RZ/H) WATER DEPTH IS 8.0INCHES. DATA FOR RUN NUMBER 46. TOTAL NUMBER OF RUNS IS 23. WAVE PERIOD IS 1.755SECONDS. DIMENSIONLESS PARAMETERS ARE

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	⋝⋍	0044-W 46-4900 46-4900	ಎಂಬನಿ
	NN MA MA MA MA MA MA MA MA MA MA MA MA MA	44444 964949 9600000 -004849	1444 0434 0304 0304

S I 4		
RATIO(=H/A)	F. COEF	00000000 •••••••• 004444444444444444444
TO RADIOS SOLOS	F. COEF	
	H/2A	00000000 •••••••• •••••• ••••• •••• ••
ARE NO	# 0 % ∃ > <b>,</b>	000000004 046644 040000044
AR AMETERS ,	FORCE (X)	OQHHHHWWW •
∾ σ	MAY	OOHHHNNUUU *******************************
DIMENSIONLES	SEGMENT NUMBER TMBER	4444444 

18166.019 18 5.5000 NON-DIMENSIONAL PERIOD( #GT \*\* 2/H) WATER DEPTH IS 11.0INCHES. DATA FOR RUN NUMBER 51. TOTAL NUMBER OF RUNS IS 23. WAVE PERIOD IS 2.175SECONDS. DIMENSIONLESS PARAMETERS ARE

F. (COEF	00000000000000000000000000000000000000
F. COEF	000000000000 0-000400000 0-00040000000000
H/2A	C0000000000444 
A の A の >> 日 >>	
FORCE XX	0000001111111000000000000000000000000
WA VE HE IGHT	000
SEGMENT	らちちちちちちちちちちちちちちちちちちららしていることでは、ままままままままままままままままままままままであっている。
	EGMENT WAVE FORÇE FORÇE H/2A F. COEF F. ÇÛE Umber Height (X)

WATER DEPTH IS 11.0INCHES. NUMBER OF RUNS IS 23. TOTAL 2.020SECONDS. DIMENSIONLESS PARAMETERS ARE DATA FOR RUN NUMBER 52. WAVE PERIOD IS

IS 5.500		
OD(=GT**2/H) RATIO(=H/A)	F. COEF	00000000000000000000000000000000000000
TONAL PERI TO RADIUS	F. COEF	0000000004444 04000000000444 040000000444
O D D D D D D D D D D D D D D D D D D D	H/2A	00000000000HHH  •••••••••••••
ARE NON.	FORCE ( y )	0000000000HHH 0000000000HHH 000HM4M7000HM40 WFWWM0004FM44FM
AMETERS	FORCE (X)	00000HHHHNNNNW
ULESS PAR	WAVE HE IGHT	000
DIMENSIONL	SEGMENT NUMBER	80000000000000000000000000000000000000

NON-DIMENSIONAL PERIOD(=GT\*\*2/H) IS124.038 DEPTH TO RADIUS RATIO(=H/A) IS 5.5000 WATER DEPTH IS 11.0INCHES. DATA FOR RUN NUMBER 53. TOTAL NUMBER OF RUNS IS 23. WAVE PERIOD IS 1.880SECONDS. DIMENSIONLESS PARAMETERS ARE

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	•
F. COEF (Y)	00000000000000000000000000000000000000
F. COÉF (X)	
H/2A	00000000000000000000000000000000000000
FORCE (Y)	000000000HHHH-
FORCE (X)	0000HHHHHUUUUU 44000UU00UU00U 644HDUPO000000
WA VE HE IGHT	00044400000000444 44004400000000000000
SEGMENT NUMBER	NNNNNNNNNNNNNNNNNNNNNNNNNNNNNNNNNNNNNN

F. COEF	00000000000000000000000000000000000000
F. COEF	00000000000000000000000000000000000000
H/2A	00000000000000000000000000000000000000
FORCE (Y)	00000000000000000000000000000000000000
FORCE (X)	
WA VE HE I GHT	0000111000000000000000000000000000000
SEGMENT NUMBER	<b>™™™™™™™™™™™™™™™™™™™™™™™™™™™™™™™™™™™™</b>

1\$245.986 1\$5.5000 NON-DI MENSIONAL PERIOD(#GT\*\*2/H) DEPTH TO RADIUS RATIO(#H/A) WATER DEPTH IS 11. JINCHES. DATA FOR RUN NUMBER 55. TOTAL NUMBER OF RUNS IS 23. WAVE PERIOD IS 2.647SECONDS. DIMENSIONLESS PARAMETERS ARE

F. COEF	00000000000000000000000000000000000000
F. COEF	HONGERNOGENS
H/2A	00000000000000000000000000000000000000
FORCE (Y)	00000000000000000000000000000000000000
FOR XX E	000000HHHHHHU •••••••••••••••••••••••••••••••
WAVE HE IGHT	40000000000000000000000000000000000000
SEGMENT NUMBER	RENEWENDERS SERVERS SE

A TOTAL STATE OF THE STATE OF T

NON-DIMENSIONAL PERIOD(=GT\*\*2/H) DEPTH TO RADIUS RATIO(=H/A) WATER DEPTH IS 11.0 INCHES. TOTAL NUMBER OF RUNS IS 23. WAVE PERIOD IS 1.752SECONDS. DIMENSIONLESS PARAMETERS ARE DATA FOR RUN NUMBER 56.

F. COEF	00000000000000000000000000000000000000
F. COEF	00000000000000000000000000000000000000
H/2A	-0000000000- -0000000000- -00000000000
FORCE (Y)	00000000000000000000000000000000000000
FORCE (X)	0000
WAVE	OOHHHMMMMMM4 *****************************
SEGMENT NUMBER	<i><b>ԽԽԽԽԽԽԽԽԽԽԽ</b> <b>4</b>04040040404 <b>6</b>0000001444 <b>4</b>044040€00144</i>

and the second second seconds in the second second

NON-DIMENSIONAL PERIOD(#GT\*\*2/H) IS192.910 DEBTH TO KADIUS RATIO(=H/A) IS 7.0000 WATER DEPTH IS 14.0INCHES. DATA FOR RUN NUMBER 71. TOTAL NUMBER OF RUNS IS 23. WAVE PERIOD IS 2.645SECONDS. DIMENSIONLESS PARAMETERS ARE

18		
RAT IO (=H/A)	F. COEF	00000000000000000000000000000000000000
TO KADIUS	F. COEF	00000000000000000000000000000000000000
0 <b>E P 1</b> H	H/2A	00000000000 0400000000044 040000000000
	FORCE (Y)	00000000000000000000000000000000000000
	FORCE (X)	0000000044444444 ••••••••••••••••••••••
	WA VE HE IGE T	00044440000m4444 0000000000000000000000
	SEGMENT NUMBER	トレイトトレイトトトトト 1 1 1 1 1 1 1 1 1 1 1 1 1

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F. COEF	00000000000000000000000000000000000000
F. COEF	00000000000000000000000000000000000000
H/2A	00000000000000000000000000000000000000
FORCE (Y)	0000000000 0000404000000044 400000000044
FORCE (X)	000000000 
WAVE HE IGHT	COOOHHHHUUUUU44R 
SEGMENT NUMBER	

NON-DIMENSIONAL PERIOD(#GT\*\*2/H)
DEPTH TO RADIUS RATIO(#H/A) WATER DEPTH IS 14.0INCHES. TOTAL NUMBER OF RUNS IS 23. WAVE PERIOD IS 2.170SECONDS. DIMENSIONLESS PARAMETERS ARE DATA FOR RUN NUMBER 73.

F. COEF	00000000000000000000000000000000000000
F. COEF	0000000000000 
H/2A	
FC \$C \$C	00000000000000000000000000000000000000
FORC E	00000044444400000000000000000000000000
WAVE HEIGHT	OOHHHHUUUUU444% 4FOUNDHHUDUAONAH WHONFOA4004000
SFGMENT NUMBER	レフトファイファイファイファイン ひろうろうろうろう しょうしょうしょうしょく しょうしょく しょうしょく しょうしょう しょうう しょう

A SECTION OF SECTION SECTIONS OF SECTION SECTIONS OF SECTION SECTIONS OF SECTION SECTIONS OF SECTION S

WATER DEPTH IS 14.0 INCHES. DATA FOR RUN NUMBER 74. TOTAL NUMBER OF RUNS IS 23. WAVE PERIOD IS 2.022SECONDS. DIMENSIONLESS

18118	-	
D(#G ##Z/H) RATIO(#H/A)	F. COEF	00000000000000000000000000000000000000
TO RADIUS	F. COEF	00000000000000000000000000000000000000
	H/2A	00000000004444 
ARE NO	FJRCE (Y)	000000000000 000
AMET ERS	円 スペン 品 (	
S PAR	WAVE HEIGHT	OOHHHUUUUU44777 TOOHUUUUUHHUUU4477 TOOHUUUNTHUUUUU4477 NOO41004000000
oimension es	SEGMENT NUMBER	77777777777777777777777777777777777777

18.72		
RATIO(#H/A)	F. COEF	00000000000000000000000000000000000000
TORADIUS	F. COEF	00000000000000000000000000000000000000
O T T T	H/2A	00000000000000 •••••••• 0-40/44/00/0000-0-0 0/0-40000000000000000000000000000000000
	FORC SY	00000000000004444 0004484488000448 48786489848
	FORC XXC	0000000HHHHHHU •••••••••••••••••••••••••••••••
n n	WAVE HEIGHT	000
	SFGMENT NUMBER	トレイナイイイイイイイイイイ おどからからならならならならなって 000000000101111111 1244500001011111111111111111111111111111

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		15152 1 S 199
NUMBER OF RUNS IS 23.	WATER DEPTH IS 18.0INCHES.	NON-DIMENSIONAL PERIOD("GT**2/H) IS152 DEPTH TO RADIUS RATIO("H/A) IS 9.
TOTAL		
DATA FOR RUN NUMBER 91. TOTAL NUMBER OF RUNS IS 23.	WAVE PERIOD IS 2.665SECONDS.	DIMENSIONLESS PARAMETERS ARE

•		
	F. COEF	00000000000000000000000000000000000000
	F. COEF	00000000000000000000000000000000000000
	H/2A	00000000000 
	FORCE (Y)	00000000000000004 00004404040000000004 446440000000000
	ACX BOX	0000000004444- 
	WA VE HE IGHT	004444408700000000000000000000000000000
	SEGMENT	00000000000000000000000000000000000000

NON-DIMENSIONAL PERIOD(#GT##2/H) IS117.684 WATER DEPTH IS 18.0INCHES. DATA FOR RUN NUMBER 92. TOTAL HUMBER OF RUNS IS 23. WAVE PERIOD IS 2.342SECONDS. OTMENSIONLESS PARAMETERS ARE

F. COEF	0000000000000 0000
Ex) COEF	0:)0000000000000 0
H/2A	00000000000000000000000000000000000000
FORCE FORCE	00000000000000000000000000000000000000
FOR XX E	000000044444444 40450000000000000000000
WA VIII HE TGHT	〇〇ユエオグススタラライムから ・・・・・・・・・・・・・・・・・・・・・・・・・・・・・・・・・・・・
SEGMEN NUMBER A	00000000000000000000000000000000000000

The state of the s

13126.640 WATER DEPTH IS 18.0 INCHES. TOTAL NUMBER OF RUNS IS 23. 2.430SECONDS. DATA FOR RUN NUMBER 93. WAVE PERIOD IS DIMENS

# . COE#	00000000000000000000000000000000000000
F. COEF	COCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCCC
H/2A	0000000000000000000000000000000000000
FORCE TY	00000000000000000000000000000000000000
800 5 5 5 6 7 7	
EAV Fin Ton F	COHHHUNDSUM (SNR)  ***********************************
SEGMENT	もならならならない ひららりからな はいさいないないないないないでした このしのこういい メリュニーエ えっちょうできない こうできない しょうしょう しゅうしゅう しゅうしゅう しゅうしゅう しゅうしゅう しゅうしゅう しゅうしゅう しゅう
	EGMENT WAVE FORCE FORCE HAZA F. COEF F. COE

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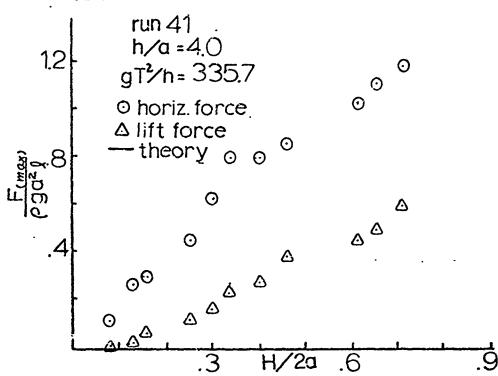
NON-DIMENSIONAL PERIOD(#GT\*#2/H) IS 94.355 WATER DEPTH IS 18.0 INCHES. DATA FOR RUN NUMBER 94. TOTAL NUMBER OF RUNS IS 23. 2.097SECONDS. DIMENSIONLESS PARAMETERS ARE WAVE PERIOD IS

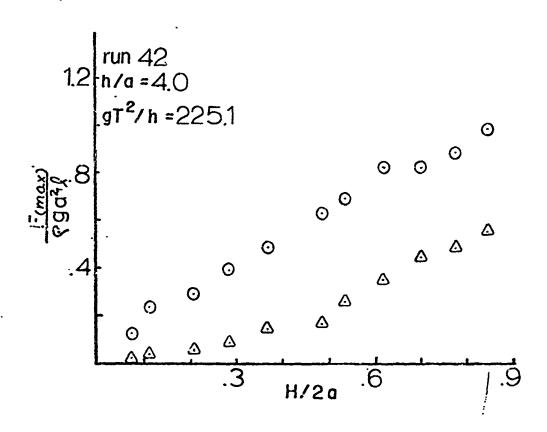
F. COEF	00000000000000000000000000000000000000
(X)	00000000000000000000000000000000000000
H/2A	000000000044444 •••••••••••••••••••••••
FORCE (≺)	0000000000000444 • • • • • • • • • • • • • • • • • • •
7 0 0 3 0 3 3	00000004444000000000000000000000000000
MAA TEN TOTH	OOH4466555555555555555555555555555555555
SANEN'T	00000000000000000000000000000000000000

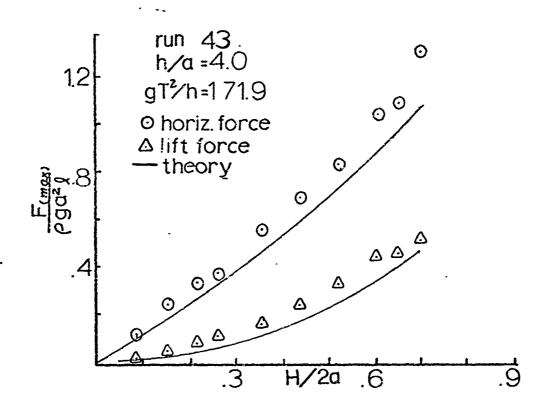
NON-DIMENSIONAL PERIOD(=GT\*\*2/H) DEPTH TO RADIUS RATIO(=H/A) WATER DEPTH IS 18.0 INCHES. DATA FOR RUN NUMBER 95. TOTAL NUMBER OF RUNS IS 23. WAVE PERIOD IS 1.957SECONDS. DIMENSIONLESS PARAMETERS ARE

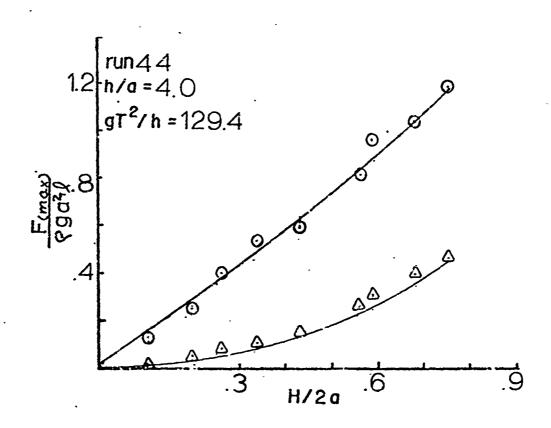
	·
F. COEF (Y)	00000000000000000000000000000000000000
F. COEF	000000000000 000000000000000000000000
H/2A	00000000HHHHHH 
FORCE (Y)	00000000000001HH 0001000000000000000000
FORCE (X)	0000000HHHINUNN ••••••••••••••••••••••••••••••••••
WAVE HEIGHT	OHMHHWWW44NW40'- NO410048N041N84 84N4HOOOCOOCOO
SEGMENT NUMBER	

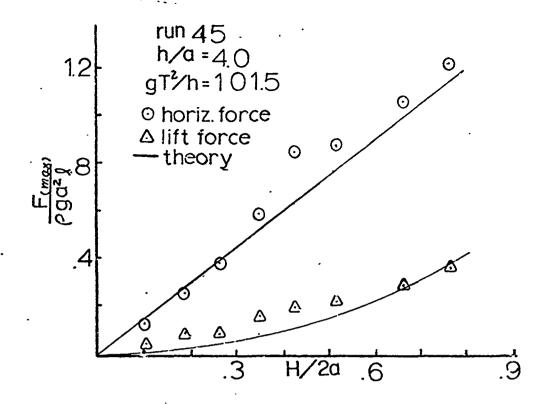
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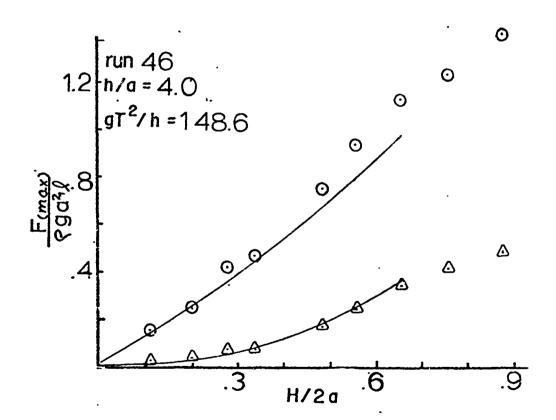




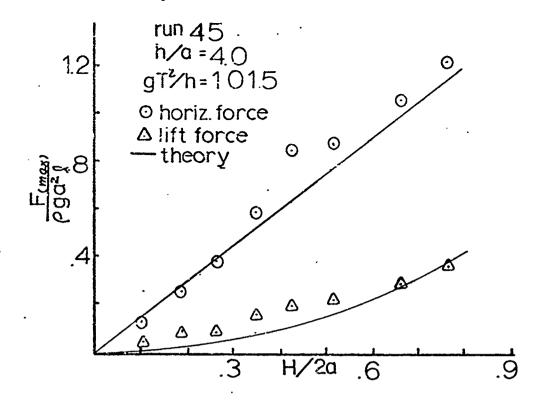


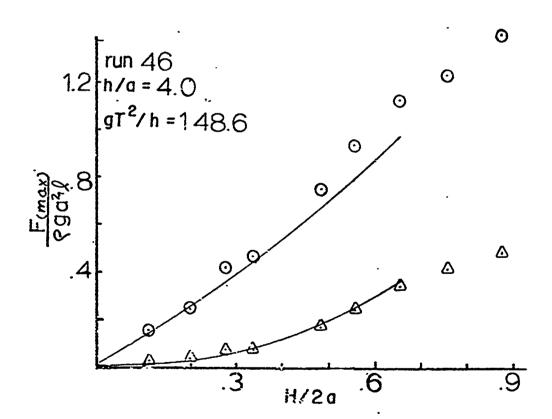


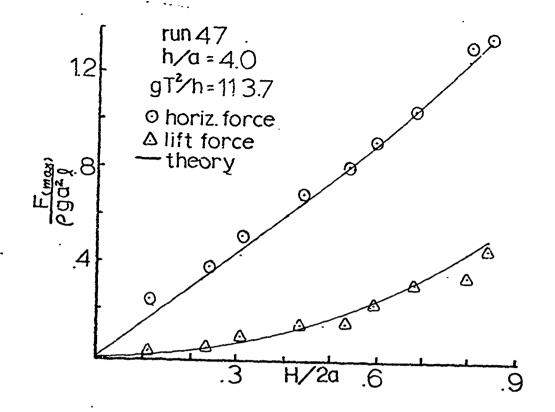


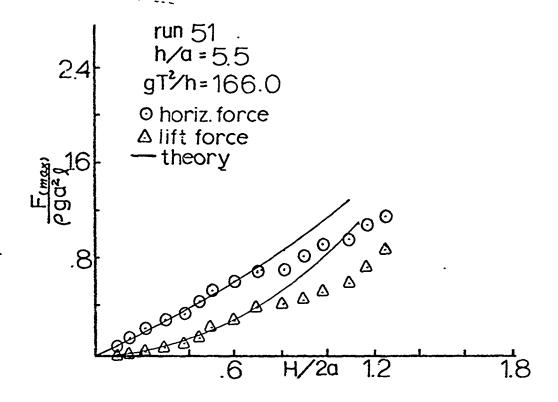


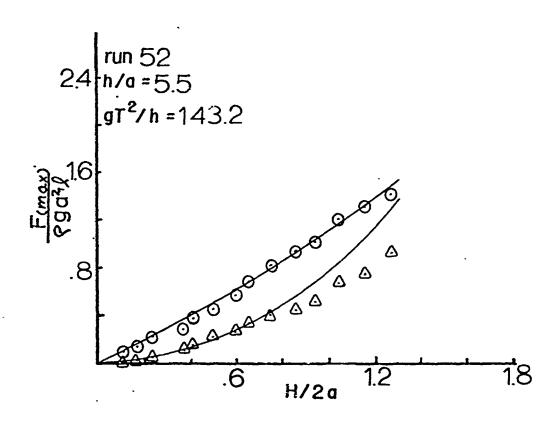
THE PROPERTY OF THE PROPERTY O

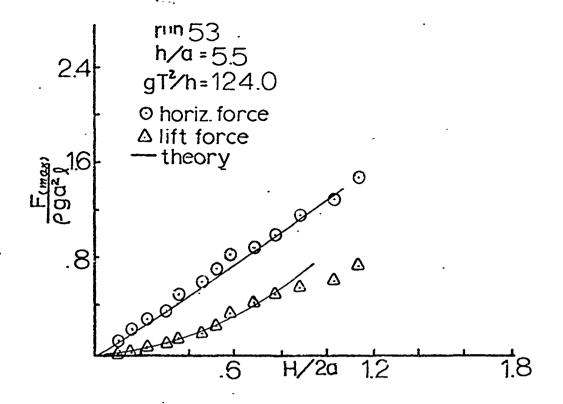


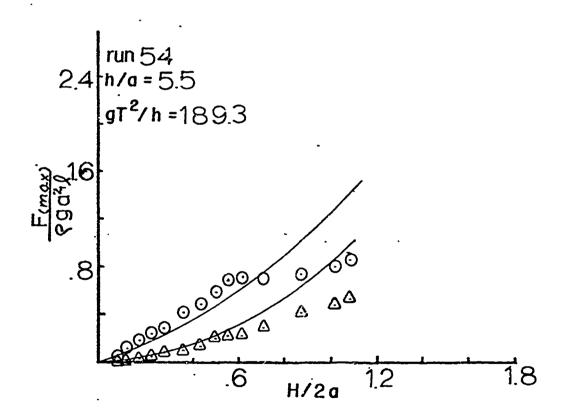


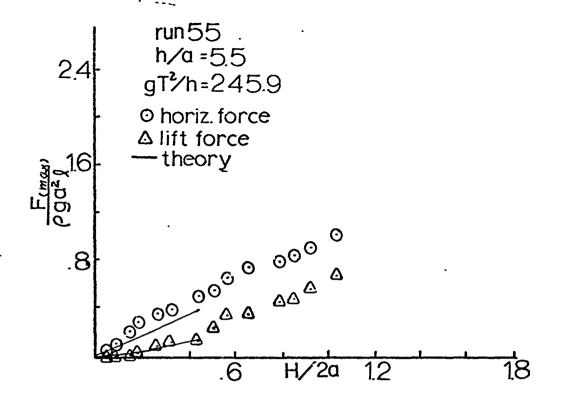


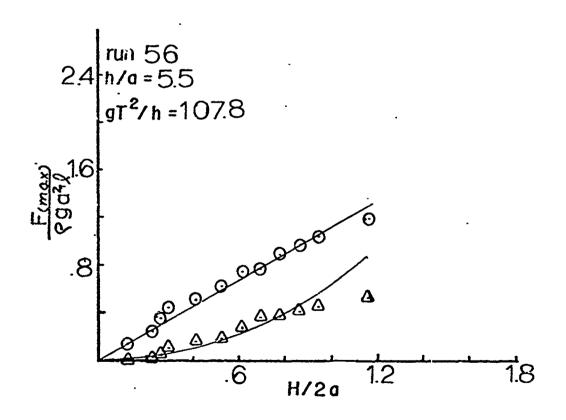


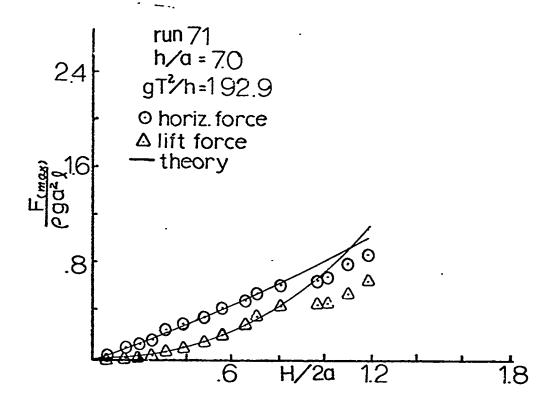


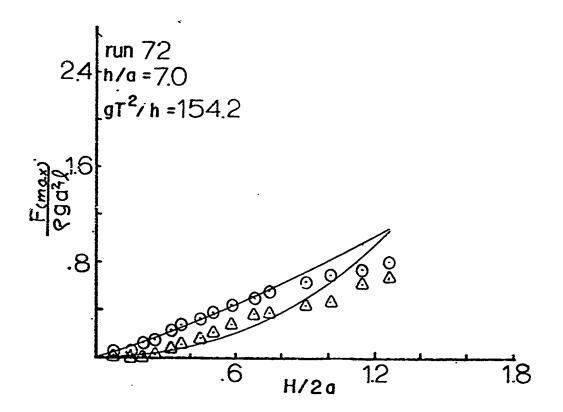


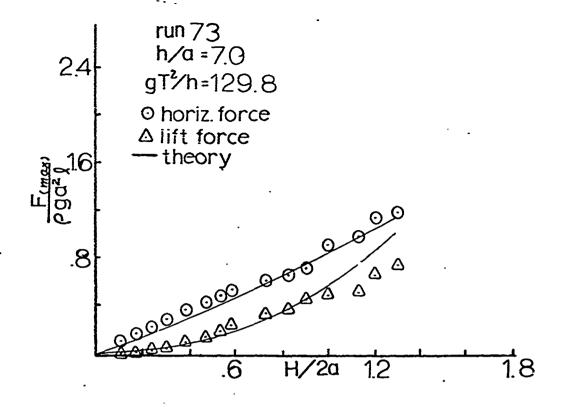


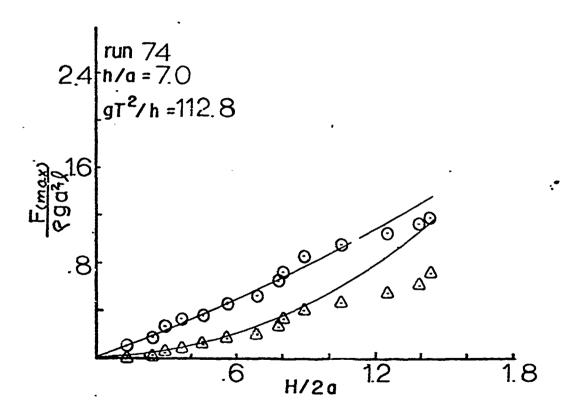


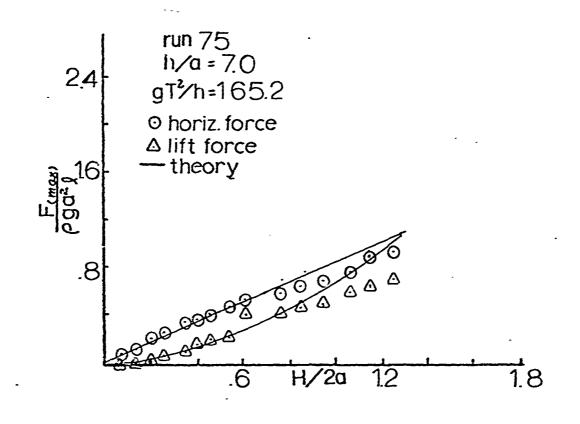


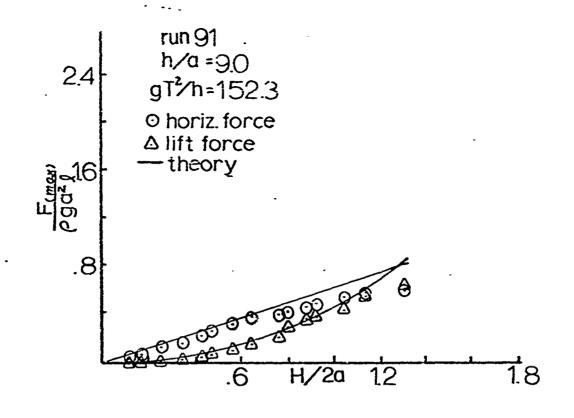


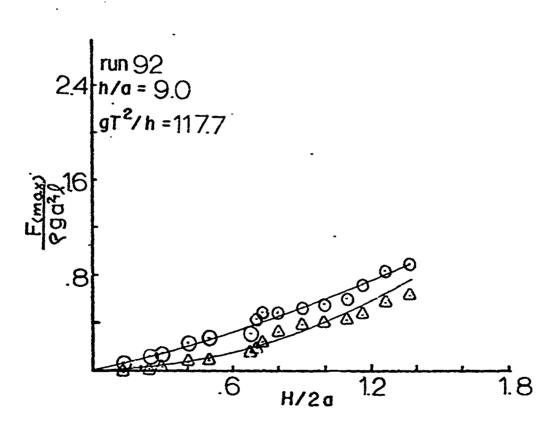


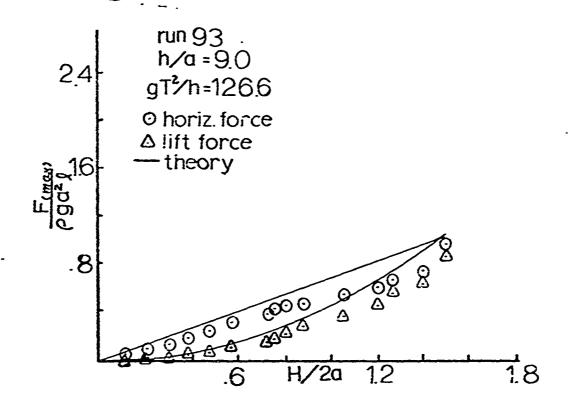


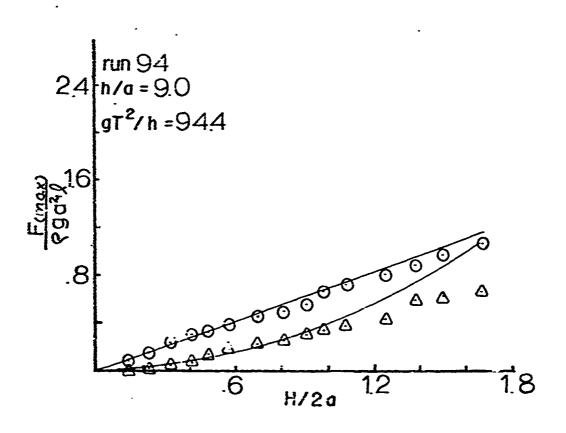


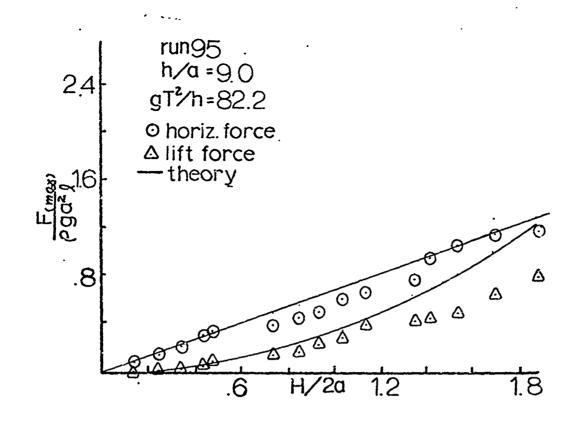












## APPENDIX D

FORCES ON A HORIZONTAL CYLINDER DUE TO NON-LINEAR WAVES

THESIS TOPIC

COMPUTER PROGRAM

NAVAL POSTGRADUATE SCHOOL

DECENBER, 1972

FRED HERMAN GEHRMAN JR.

THIS PROGRAM COMPUTES THE FORCES ON A BOTTOM-MODRED RIGHT CIRCULAR

CYLINDER DUE TO WAVE MOTION USING STOKE'S FIFTH CROER GRAVITY WAVE

THEDRY. THE INPUTS TO THE PROGRAM ARE

TN=NGNDIMENSIONAL PERIOD PARAMETER=G\*(T\*\*2)/DEPTH DN=DEPT+/CYLINDER RADIUS HN=WAVEHEIGHT/CYLINDER DIAMETER

PARAMETERS OF THE PROGRAM ARE OTHER

H"WAVEHE I GHT / DEPTH

TI=TIME/PERIOD

X=HORIZONTAL DISTANCE/WAVELENGTH

D=2\*PI\*DEPTH/WAVELENGTH

BETA=D/DN=2\*PI\*CYLINDER RADIUS/WAVELENGTH

CL=LIFT COEFFICIENT

CM≅ADDED MASS COEFFICIENT

G=GRAVITATIONAL CONSTANT

```
00000320
00000330
00000340
                                                                                               00000140
                                                                                                                                                                                                                                                                                00000360
                                                                                   00000130
                                                                                                                  00000167
                                                                                                                                                                                                                             00000310
                                                                                                                                                                                                                                                                    00000350
                                                                                                                                                                                                                                                                                                   00000
                                                                                                                                                                                                   *0**,6x,*p*,11x,*u(3)*,11x,*u(4)*,11x,*u(5)*,11x,*C*,11x,
*1CS*,11X,1H*)
*14) bx;U(3),U(4),U(5),C,L0,CS,H
                                                                                                                                                100), PR(100), DE(100), DUM1(100)
                                                                                                                                                                                                                                                                                                    POINTS
                                                                                                                                                                                                                                                                                                    DATA
                                                                                                                                                                                                                                           BETA=D/DN
WRITE (6:298) BETA
FORMAT (10:.9ETA=.,1PG14.6)
                                                                                                                                                                                                                                                                                                    NUMBER
                                                                                                                   STAKESS
                                                                                                                                                                                                                              COMPUTE WAVE PROFILE
                                                                                                READ (5.1) TN.HN.DN
HEZ.O*HN/DN
  IMPLICIT REAL#8 (A-
REAL#4 DUMI, DUM 2, DU
DIMMENSION Y (5) + U(5)
BDUMA(100+1) + U) OT(10
CM 2-28
CL 4-48
GE 32-2
PIE 3-141592653
PIE 3-141592653
                                                                                                                                                                                                                                                                     INITIALIZE VALUES
                                                                                                                                                                                                                                                                                                    K DETERMINES THE
                                                                                    READ IN VALUES
                                                                                                                   ADJUST TN FOR
                                                                                                                                                                                                                                                                                  X#0.0
                                                                                                                                                                                                                                                      298
```

U

		~ ~	
		J=K+1 DO 200 I=1 • K TH1=P12* (X+T1)	0000 0000 0000 0000 0000 0000 0000
U		COMPUTE PHASE ANGLE	**********
		DE(1) = (180.0/PI) * TH1	240000
ပ		SET UP FOR UTPLOT	## 00000 ## 00000
		DUMI(I) #DE(I) [LISH 2 .0 4 LT] [TISH 3 .0 4 LT] [TISH 4 .0 4 LT] [TISH 5 .0 4 LT] [TISH 5 .0 4 LT]	0 0000 0 0000 0 0000 0 0000 0 0000 0 0000 0 0000 0 0000 0 0000
ပ		COMPUTE WAVEPAOFILE	20000
	⋖	PR(1) = (1.0/0) + (Y(1) + DCOS (TH1) + Y(2) + DCOS (TH2) + Y(3) + DCOS (TH3) + Y(4) + DCOS (TH4) + Y(5) + DCOS (TH5))	000000000000000000000000000000000000000
ပ		SET UP FOR OSPLOT	
		DUM2(I.1)mPR(I)	
U		STEP TIME	000000
•	200	Timil+1.0/J CONTINUE	00000 00000 00000
ပ		PLOT OF VALUES	
		0	00900000
ပ		JEL PLOT	00000010
·	500	WRITE (6,500) FORMAT (101,20X, WAVE PROFILE) WRITE (6,501) FORMAT (101,20X, X - PHASE ANGLE, DEG:Y - NONDIMENSIONAL WAVEHEIGHT	00000000000000000000000000000000000000
	502	(TE' (6,502) TN, DN, HN, D (MAT ('0', 'TN"', IPG14.4, 'DN"', G14.4, 'HN"', G14.4, 'D"', G	0000 0000 00000 00000 00000
ပ		BLE OF VAL	06900000
		WRITE (6.201)	0000000

د	201	FORMAT ('1',13%,'PHASE ANGLE',10%,'Y VALUE") WRITE '6'202) (DE(1),PR(1),I=1,K) FURMAT ('0',8%,1PD14,4,6%,014,4) DETERMINE VELOCITY VS. DEPTH	0000071 0000072 00000073 0000073
	299	ETAMPR(1)*0 STARG=0+ETA WRITF(6:299) ETA FORMAT(10:9) ETA*, F16.8)	0000075 00000076 00000000
ပ		REINITIALIZE X AND TI	000000
		111 × 0 × 0 × 0 × 0 × 0 × 0 × 0 × 0 × 0	000000 000000 000000 000000 000000 00000
ပ		J DI IFRMINES INF MIMARR OF DATA POINTS	000000
		08.30	00000
ပ		Z = NON-DIMENSIONAL DISTANCE FROM BUITOM=570	0000089
		Z=0.0 DO 400 I=1.100 K=1 S(1)=2	06 the Purity 00 00 00 00 00 00 00 00 00 00 00 00 00
ပ		⊃ 	<b>4600000</b>
			0000000 0000000 0000000 0000000 0000000
Ľ		COMPUTE VELOCITY	0000103
	300	VEL(TI =C*(U(I) *DCOSH(ARGI)*DCOS(THI)+U(2)*DCOSH(ARGZ)*DCDS(TH2)+ AU(3)*ECOSH(ARGS)*DCOS(THB)+U(4)*DCOSH(ARG4)*DCOS(TH4)+U(5)* BOCOSH(ARGS)*DCOS(THS))	00000134 00000105 0000106

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ပ		SET UP FOR USPLOT	00001000
ပ	004	DUM1 (1) #VEL (1) STEP DEPTH 2#Z+1 O/J CON7 INUE	00000000000000000000000000000000000000
ပ		PLOT OF VALUES .	00001120
	401	DUMI(K) 30.0 VEL(K) 00.0 CALL OSPLOT (DUMI, DUM2, K, K, 1, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0,	50001130 00001140 00001150
'n		J.ABEL PLOT	00001160
	009	WRITE (6.600) FORMAT (1000) - ORIZONTAL VELOCITY DISTRIBUTION ON VERTICAL SECTION COO A THROUGH WAVE CREST()	000011700000000000000001190000
	109	WRITE (6,501) FORMATE (0,000) WATER DEPTH X - HORIZONTAL PARTICLE	0120 0120 0120
	603	WRITE (6,603) TN.DN.HN.D FORMAT (10., TN= , 1PG14.4, ON= , G14.4, HN= , G14.4, C14.4, C1	0123
ပ		TABLE OF VELOCITY VALUES	00001250
	403	WRITE (6:402) FORWAT (1:0.10x.0) FORWAT (1:0.10x.0) FORWAT (1:0.10x.0) FORWAT (1:0.10x.0) FORWAT (1:0.10x.0)	000001260 000001270 00001280 00001280
ပ		COMPUTE FURGES ON CYLINDER	00001300
ပ		REINITIALIZE PARAMETERS	01610000
		公里O。O TI = O。O	00001320
ပ		-	00001340
		I H H H H H H H H H H H H H H H H H H H	00001350 00001350 00001350 00001350
ť		DETERMINE FUNCTION ARGUMENTS	00001390

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	ARGULA CO + D SI N (RAD) > ARGULA CO + D SI N (R	00000 00000 000000 000000 000000 00000 0000
	T	0000 4444 8466
	CC	ひひらひひ イコムスー 今ろうがあ ゆひよびあ
ပ	UDT = NONDIMENSIONAL X ACCELERATION	00001540
113	30 UDT#(U(1)*DCOSH(ARG1)*DSIN(TH1)+U(2)*DCOSH(ARG2)*DSIN(TH2)*2.+ AU(3)*DCOSH(ARG3)*DSIN(TH3)*3.+U(4)*DCOSH(ARG4)*DSIN(TH4)*4.+U(5)* BOCOSH(ARG5)*DSIN(TH5)*5.)*D*CS	00001550 00001560 00001560
ပ	FV=NONDIMENSIONAL INERTAIL FORCE	00001580
	TV#DN*UVELS*CL DUM9 (II • I) # IV	00001590
ပ	FHN=NONDIMENSIONAL HORIZONTAL FORCE	01910000
	FIN(11) #P1 # (1, 0 +CM) #UDT DUMZ (11, 1) ##HN(11) DE(11)	00000 000001620 00001640 00001640
ပ	K DETERMINES THE NUMBER OF POINTS ALONG THE CYLINDER	09910000
	KngO	00001670
ပ	INITIALIZE VALUES	00001680
ပ	PN "NONDIMENSIONAL PRESSURE BASED ON BERNOULLI EQUATION	06910000
	•	00001700
ပ	SET UP FUNCTION ARGUMENTS	00001730
	ARG1 = 0"T A# (1.0:051N(RAD))+PI/K	00001740

00000000 00000000 00000000 00000000 0000	00001840	00000 000001860 00001870 00001870	00001890 00001900 00001910	00001920	00000 00000 000000 000000 000000 000000	06610000	000000000000000000000000000000000000000	00002030	00 420	2	00000 00000 00000 00000 00000 00000	<u> </u>
ARGOUST ARGOUNT ARGOUN		0 TS # C S # (U(1) * 0 C O S # (ARG1) * 0 C O S (TH1) (2) * 0 C O S # (ARG3) * 0 C O S (TH1) (0 S # (ARG3) * 0 C O S (TH3) + U(4) * 0 C O S # (ARG3) * 0 C O S (TH3)	######################################	VVEL #NOND! P	>COD>	TERATE ARC	RADERACTPIZ/K CONTINUE CUM3(II.2) #PN	エラント 当していた	+ \1 -	ITERATE THROUGH 360 DEGREES	000 CONTINUE 0/111 CALL OSPLOT (2UM1.0UM2.1.1.1.0.000.)	OKNAT (16
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a O E	(RTA-0.01) 3.1.1	(RECTION 00002500	(4 T/HT) 00002510 00002520 00002520	ITFY CONVERGENCE UC002530	TE (9.4) RTA DID NOT CONVERGE IN 500 ITERATIONS.ERROR IS., P16.8) 000025550 TERATIONS.ERROR IS., P16.8) 000025550 TERATIONS.ERROR IS., P16.8) 000025570	2 2 2 3	&0 J±1,500 0002590	UPB 40	COCCOCCOCCOCCOCC  *********************	PUTE COEFFICIENTS 00002610	n 3 . 0 % (
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00000- +04		0	HSON ITERA	(A+(A##3	ONVERGENCE	53.49.49			VERSENCE	**************************************	PER100	*(1.0+A2	onvergence	0) 65,89	
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50 DED40.100#ER 50 CON11NUE 50 CON11NUE	ERIFY CONVERGENCE	ORMAT (************************************	DETERMINE & COEMPICIENTS	# COL # (2 * O # CO 2 + 1 * O ) / (4 * O # SO 3 ) # CO 1 # (2 7 % * ) # CO 3 + 5 0 4 0 4 CO 6 + 1 9 2 * O # CO 4 + 3 2 2 * O # CO 2 + 2 1 * O ) / (3 8 4 * O # SO 9 ) # CO 1 # (7 6 9 * O # CO 1 0 - 4 4 8 * G * CO 8 + 4 8 * O # CO 4 + 1 0 6 * O # CO 2 - 2 1 * O ) / 4 * O # SO 9 * (6 * O * CO 2 - 1 * O )	ETERMINE WAVE CELERITY.C	"DTANH[D] "(-1.0)/(4.0*SO1*CO1) "(12.0*CO8+36.0*CO6-162.0*CO4+141.0*CO2-27.0)/(192.0*CO1*SO9)	ETERMINE WAVE EQUATION COEFFICIENTS	1 = 1 - 0   3   1   1   2   2   2   2   2   2   3   3   3   3

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0000370
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', A13, A15, A22, A24, A33, A35, A44
                                                                          *6X;'D',11X,'B22',11X,'B24',11X,'B33',11X,'B35',11X,

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DX,B22,B24,B33,B35,B44,B55,A11

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